



PROPOSED MIXED USE DEVELOPMENT 137-141 WALDRON ROAD, CHESTER HILL

Traffic and Parking Assessment Report

21st April 2017

Ref: 15023

Prepared by

Terraffic Pty Ltd

Traffic and Parking Consultants



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1. INTRODUCTION

This report has been prepared to accompany a Development Application (DA) to the Canterbury Bankstown Council for a proposed mixed use development at 137-141 Waldron Road, Chester Hill (Figures 1 and 2).

The proposed consolidated development site is located on the south-eastern corner of the Waldron Road / Campbell Hill Road intersection. The site has a total area of 3,351m² with frontages of approximately 85m to Waldron Road and 31m to Campbell Hill Road. A site survey prepared by Structure Surveying is reproduced in the following pages.



Aerial photograph of the site

Existing Site Development

The existing site development comprises a single storey retail building with a floor area of approximately 1250m^2 . The retail shop is currently occupied by Ivan's Butchery & Delicatessen. The site also contains a former Volume Plus service station with a site area of approximately 830m^2 and associated convenience store with an area of approximately 175m^2 . The service station has been closed since 2009 and is currently used to store parked cars.



As can be seen on the aerial photograph, the existing retail shop is served by an at-grade Right of Carriageway (ROW) at the rear of the site that can accommodate approximately 30 parked cars. The ROW also provides an egress route for vehicles parking at the rear of the neighbouring retail premises at 129 Waldron Road.

The former Volume Plus Service Station gained vehicular access to Waldron Road via a 12m wide entry driveway and 10m wide exit driveway to Campbell Hill Road. As the proposed development will make these driveways redundant, the on-street parking capacity will increase by up to 4 parked cars along the site frontage.



Looking east along the ROW from Campbell Hill Road

Proposed Site Development

The development proposal involves the demolition of the existing site development and construction of a mixed use development comprising:

- 100 residential units (11 x studios, 31 x 1 bedroom units, 44 x 2 bedroom units and 14 x 3 bedroom units)
- 5 x ground floor retail premises with a combined floor area of 460m²



The proposed development is served by 2 level basement carpark containing a total of 109 spaces comprising 11 retail tenant spaces, 78 resident spaces and 20 visitor spaces.

Vehicular access to the basement carpark is via a 6.1m wide combined entry/exit driveway to Campbell Hill Road. The basements are connected by an internal ramp that can accommodate two-way flow.

In addition to the 109 spaces located in the basement carpark are 36 surplus parking spaces located on Ground Level. Vehicular access to the ground level parking spaces is via the two-way ROW at the rear of the site. The ROW will continue to provide an egress route for vehicles exiting the neighbouring property to the east.

The 36 surplus spaces comprise 12 retail spaces, 4 resident spaces and 20 visitor spaces and are either located on the ROW or within the site with direct access to the ROW. <u>It should be noted that these 36 spaces are additional to the development and not required to comply with the necessary SEPP and Council parking requirements.</u> The surplus spaces will be utilised until such time that the ROW is no longer available.

In addition to the off-street car parking provision are 43 bicycle racks comprising 30 resident and 13 visitor bicycle racks. These racks are located on the ground level and upper basement level.

The 5 retail shops vary in size from 67m² to 101m² and are located along the Waldron Road frontage. The shops will be served by an off-street loading bay capable of accommodating the Australian Standard 8.8m long Medium Rigid Vehicle (MRV). The loading bay has direct access to the ROW at the rear of the site.

Plans of the development proposal have been prepared by CMT Architects are reproduced in Appendix A.



Public Transport Accessibility

The development site has convenient access to the following bus routes operated by Punchbowl Bus Co:

Route M91 Hurstville to Parramatta
Route 916 Chester Hill to Guildford
Route S2 Sefton to Granville

Route S4 Chester Hill to Fairfield via Villawood Station

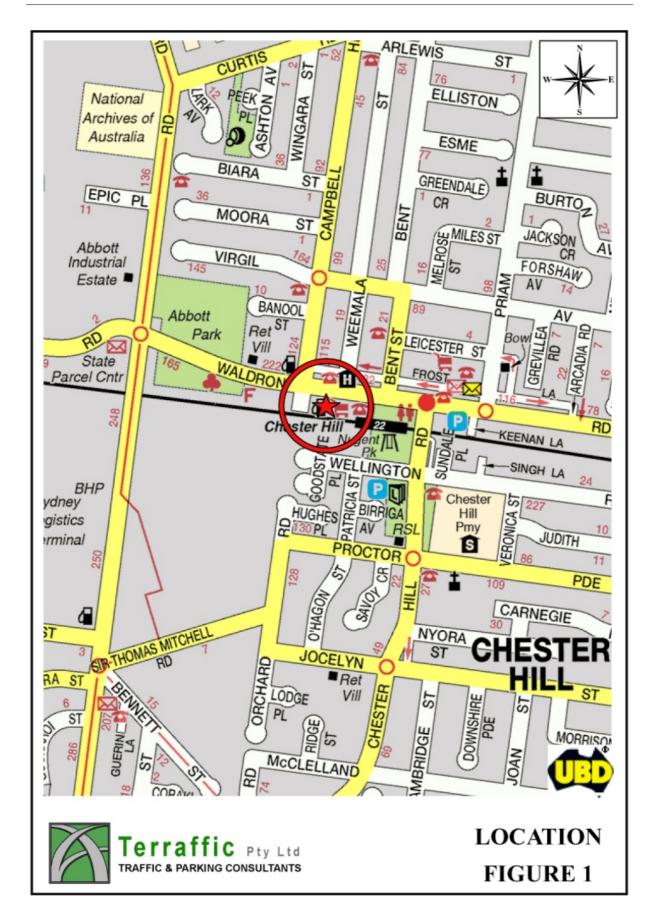


Bus Services through Chester Hill

In addition to these bus services, the development site is conveniently located 215m from the entrance to Chester Hill Railway Station on Chester Hill Road.

The purpose of this report is to assess the traffic and parking implications of the proposed development.

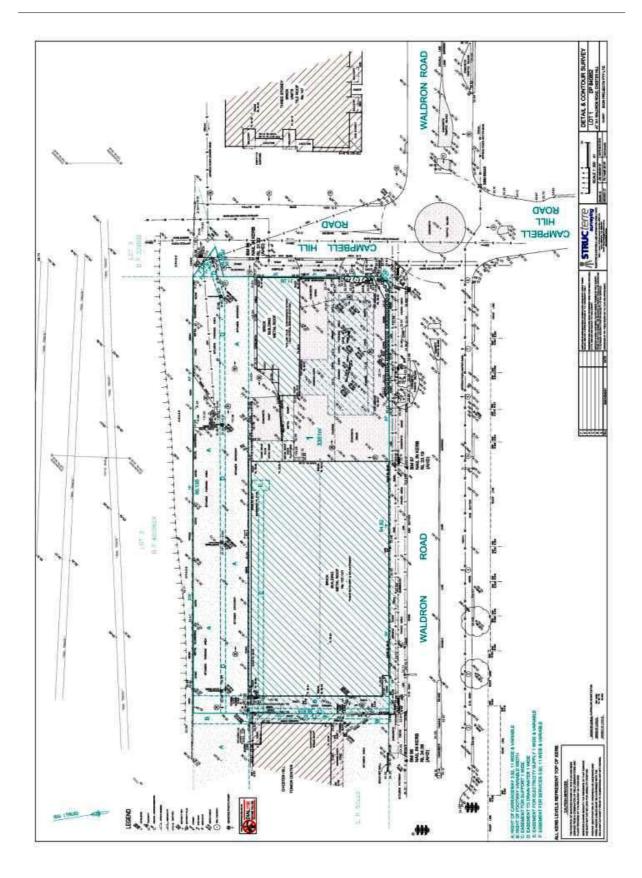












Site Survey



2. PARKING ASSESSMENT

SEPP 65 Residential Parking Requirements

The NSW Government has recently adopted the "Apartment Design Guide" which is used in conjunction with the State Environmental Planning Policy No.65 – Design Quality of Residential Flat Development (SEPP 65). Objective 3J-1 of the Design Guide states that sites within 800m of a railway station or light rail stop can satisfy the minimum parking requirements specified in the RMS "Guide to Traffic Generating Developments" (October 2002).

The RMS parking requirements for high density residential flat buildings in Metropolitan Sub-Regional Centres are as follows:

1 bedroom units
2 bedroom units
3 bedroom units
1.4 spaces per dwelling
Visitor Parking
1 space per 5 dwellings

As the development site has convenient access to Chester Hill Railway Station and numerous bus routes, the proposed development will not be providing off-street car parking for the studio apartments.

Application of those requirements to the proposed development yields a resident parking requirement of 98 car parking spaces as follows:

11 x studios @ 0 spaces per unit = 0 spaces 31 x 1 bedroom units @ 0.6 space per unit = 18.6 spaces

44 x 2 bedroom units @ 0.9 spaces per unit = 39.6 spaces

14 x 3 bedroom units @ 1.4 spaces per unit = 19.6 spaces

Total resident parking = 77.8 spaces (say 78 spaces)

100 units @ 1 visitor space per 5 units = 20 spaces

Total = 97.8 spaces (rounded up to 98 spaces)



The proposed provision of 78 resident and 20 visitor off-street car parking spaces satisfies the minimum requirement specified by SEPP65. In the circumstances the proposed resident carparking provision is considered adequate.

Council DCP Retail Parking Requirement

Part B5 - Parking of the Bankstown Development Control Plan 2014 specifies the following parking requirements that apply to the retail component of he proposed development:

Shops

Development of less than 4,000m² gross floor area - 1 car space per 40m² of gross floor area.

It should be noted that Council's parking requirement of 1 car space per 40m² GFA for retail shops is <u>identical</u> to the parking requirement specified for office floor space specified in the RMS "Guide to Traffic Generating Developments". It can therefore be assumed that:

- 1. The Council's parking requirement for retail floor space is to accommodate retail tenants only
- 2. Council is implementing a *constrained* parking policy that will effectively encourage alternative modes of transport for shoppers (such as public transport, cycling, etc) and ultimately reduce the traffic generated by retail developments

Application of the DCP parking rates to the proposed site development yields a total requirement of 11 spaces calculated as follows:

The proposed development satisfies the DCP requirement with the provision of 11 off-street to serve the retail tenants (ie long-term parking).

As noted in the foregoing, the proposed development is served by 36 surplus parking spaces located on Ground Level comprising 12 retail spaces, 4 resident spaces and 20 visitor spaces. These spaces can be utilised until such time that the ROW is no longer available.



Carpark Compliance

The carpark and access ramps have been designed to satisfy the following requirements of the Australian Standard AS/NZS2890.1-2004 – "Off-Street Car Parking":

- Class 1A (long-stay) resident and tenant parking spaces have a minimum length of 5.4m and width of 2.4m
- Class 3 (short-stay) parking spaces have a minimum length of 5.4m and width of 2.6m
- An additional 0.3m has been provided for spaces adjacent to a wall or obstruction
- Parallel spaces exceed the length requirements specified in Figure 2.5
- The access/manoeuvring aisles satisfy the minimum requirement of 5.8m
- An addition 0.5m in width has been provided to access aisles with parallel spaces parking on one side and angled parking on the other side as per Clause 2.4.4(b)(iii)
- Pavement cross-falls at parking spaces do not exceed 5% (1 in 20) in any direction
- Columns have been located in accordance with Clause 5.2 of the Standard
- The first 6m of the access from the property boundary does not exceed 5% (1 in 20)
- The two way access has a minimum width of 5.5m kerb to kerb
- Maximum ramp grades do not exceed 25% (1 in 4)
- Ramp transitions do not exceed 12.5% (1 in 8) over a distance of 2.0m
- A minimum headroom clearance of 2.2m has been provided throughout the lower basement
- Pedestrian sight lines in accordance with Figure 3.3 of the Standard have been provided

The disabled parking spaces have also been designed in accordance with the Australian Standard AS/NZS2890.6:2009 – "Off-street parking for people with disabilities" as follows:

- A 5.4m long x 2.4m wide dedicated (non-shared) parking space (Clause 2.2)
- An adjacent *shared* area that is also 5.4m long x 2.4m wide (Clause 2.2)
- A minimum headroom of 2.5m above the disabled spaces (Clause 2.4)
- Pavement cross-falls in disabled spaces do not exceed 2.5% (1 in 40) in any direction (Clause 2.3)



Loading and Unloading Facilities

Section 5 of Council's DCP specifies the following with regard to loading arrangements for mixed use developments:

- 5.2 Mixed use development must provide appropriate loading/unloading or furniture pick—up spaces. If no provision is made for the facilities, development applications must provide justification why they are not necessary.
- 5.3 Where rear lane access is not available and the commercial/retail gross floor area of a building is greater than 500m², Council requires:
 - (a) at least one off-street parking space for delivery/service vehicles; and
 - (b) additional off-street parking spaces or a loading dock depending on the size, number, and frequency of delivery/service vehicles likely to visit the premises.
- 5.4 The design of loading docks must:
 - (a) be separate from parking circulation or exit lanes to ensure safe pedestrian movement and uninterrupted flow of other vehicles in the circulation roadways;
 - (b) allow vehicles to enter and leave an allotment in a safe manner; and
 - (c) have minimum dimensions of 4 metres by 7 metres per space.

The proposed development satisfies the DCP requirements with the provision of a dedicated loading bay on ground level with direct access to the ROW. The loading bay measures 8.8m x 3.5m and has been designed to accommodate the Australian Standard 8.8m Medium Rigid Vehicle (MRV).

The ability of the MRV to access the proposed loading bay was tested using the Autodesk *Vehicle Tracking* software. The manoeuvring path of the MRV accessing the loading bay is reproduced in Appendix B and illustrates that this delivery vehicle can adequately access the loading bay in a total of three manoeuvres.



In addition to the loading dock path, Appendix B includes the entry and exit swept path between the ROW and Campbell Hill Road. As can be seen this manoeuvre can be carried out satisfactorily.

In the circumstances, it can be concluded that the proposed development has no unacceptable parking or service implications.



3. TRAFFIC ASSESSMENT

Road Hierarchy

The road hierarchy allocated to the road network in the vicinity of the site by the Roads and Maritime Services (RMS) is illustrated on Figure 3 and comprises the following:

State Roads

Regional Roads

Nil Christina Road - Waldron Road - Carlingford Street

Miller Road (south of Waldron Road)

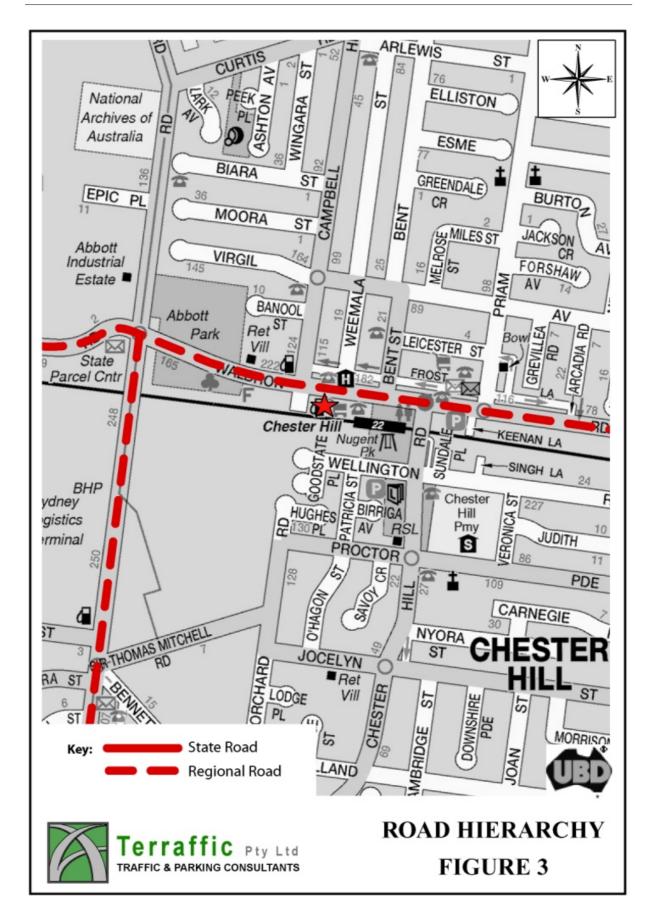
As can be seen, Waldron Road is a classified *Regional Road* performing a sub-arterial road function. It typically carries 4 traffic lanes although the kerbside lanes are generally used for parking. Waldron Road has a pavement width of approximately 13m and is restricted to a speed limit of 50km/h.

The section of Campbell Hill Road fronting the site is an unclassified Local Road with a length of approximately 40m between Waldron Road and the railway corridor. Campbell Hill Road has a pavement width of 10m with unrestricted parking permitted along both alignments. In addition to providing vehicular access to the existing site development and ROW, the residential flat building on the western side of the roadway also gains vehicular access to Campbell Hill Road.

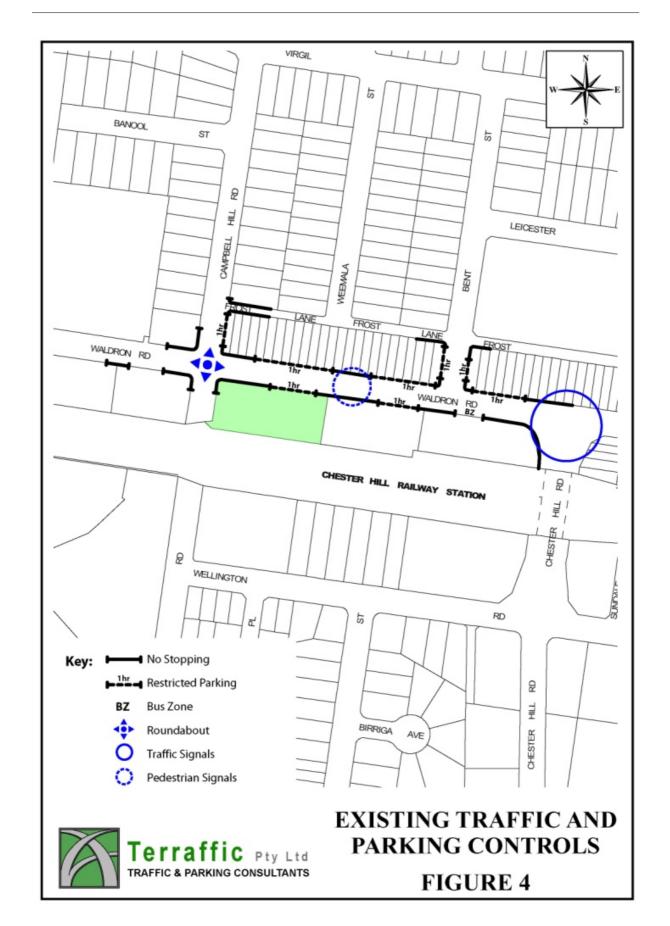
The existing traffic and parking controls on the road network serving the site are illustrated on Figure 4 and comprise:

- The ROUNDABOUT at the intersection of Waldron Road and Campbell Hill Road
- The TRAFFIC SIGNALS at the intersection of Waldron Road and Chester Hill Road
- The PEDESTRIAN SIGNALS on Waldron Road
- The 1 HOUR PARKING restrictions along Waldron Road











Existing Traffic Conditions

An indication of the existing traffic conditions on the road network in the vicinity of the site is provided by peak period traffic surveys undertaken at the Waldron Road/Campbell Hill Road intersection between 7.00-9.00am and 4.00-6.00pm on Wednesday 13th May 2015. In addition, the traffic accessing the ROW at the rear of the subject site off Campbell Hill Road was also counted during the survey periods.

The results of the traffic surveys are reproduced in full in Appendix C and reveal that:

- the morning peak period occurs between 7.45-8.45am. At that time, the two-way traffic flows on Waldron Road past the site were 1,496 vehicles per hour (vph) comprising 818vph heading eastbound and 678vph heading westbound. At that time there were 20vph on Campbell Hill Road comprising 13vph heading northbound and 7vph heading southbound
- during the morning peak, 3 vehicles entered the ROW while 6 vehicles exiting the ROW and turned right onto Campbell Hill Road
- the evening peak period occurs between 4.30-5.30pm. At that time, the two-way traffic flows on Waldron Road past the site were 1,478vph comprising 666vph heading eastbound and 812vph heading westbound. At that time there were 20vph on Campbell Hill Road comprising 13vph heading northbound and 7vph heading southbound
- during the evening peak, 7 vehicles entered the ROW from Campbell Hill Road while 6 vehicles exited the ROW

Projected Traffic Generation Potential

An indication of the traffic generation potential of the proposed development is provided by reference to the Roads and Maritime Services (RMS) *Guide to Traffic Generating Developments – Technical Direction TDT 2013-04a (August 2013).*



The traffic generation rates specified in the updated Guidelines are based on extensive surveys of a wide range of land uses throughout Sydney and regional NSW and nominate the following traffic generation rates for high density residential flat buildings:

AM Peak (1 hour) vehicle trips per unit 0.19
PM Peak (1 hour) vehicle trips per unit 0.15

As noted in Chapter 2 of this report, Bankstown Council's parking requirement of 1 car parking space per 40m^2 GFA for retail floor space is identical to the parking requirement specified for office floor space in the RMS's "Guide to Traffic Generating Developments". While the Council DCP does not specify parking allocations for retail spaces, this report assumes that the 12 spaces required by the DCP will be allocated to retail tenants only. To that end, the traffic generated by the retail floor space will be identical to the traffic generated by a typical office development, ie 2.0vtph per 100m^2 GFA.

Traffic Generating Potential of Proposed Site Development

Application of the above traffic generation rates to the residential and retail components of the development proposal yields a traffic generation potential of approximately 28vtph during morning peak period and 24vtph during the evening peak period as follows:

Morning Peak Period

100 units @ 0.19 vtph per dwelling19 vtph (4 in / 15 out) $460 \text{m}^2 \text{ retail } @ 2 \text{vtph per } 100 \text{m}^2$ 9 vtph (8 in / 1 out) \mathbf{Total} $\mathbf{28 \text{vtph } (12 \text{ in } / 16 \text{ out})}$

Evening Peak Period

100 units @ 0.15 vtph per dwelling15 vtph (12 in / 3 out) $460 \text{m}^2 \text{ retail } @ 2 \text{vtph per } 100 \text{m}^2$ 9 vtph (1 in / 8 out) \mathbf{Total} $\mathbf{24 \text{vtph } (13 \text{ in } / 11 \text{ out)}}$

The traffic generation of the proposed development should be discounted by the traffic generating potential of the existing retail premises and former service station on the site.



Traffic Generating Potential of Existing Site Development

The 2002 version of the RMS Guide to Traffic Generating Developments provides the following rate for service stations with convenience stores:

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Evening Peak Hour Vehicle Trips = 0.04 \text{ A(S)} + 0.3 \text{ A(F)}

Where A(S) = area of site (m<sup>2</sup>)

A(F) = gross floor area of convenience store (m<sup>2</sup>)
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Based on measurements taken from the site survey, the former service station has a site area of 830m² and convenience store area of 175m²GFA.

In addition, as the RMS Guidelines do not indicate a traffic generation rate for the morning peak, this assessment will assume that the morning peak generates 50% of the traffic generated during the evening peak.

Application of the RMS service station rate and the abovementioned retail shop rates, the existing site development has the potential to generate in the order of 68vtph during the morning peak and 111vtph during the evening peak as follows:

Morning Peak Period

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Service Station (0.04 \times 830) + (0.3 \times 175) \times 50\% 43vtph (22 in / 21 out)
1,250m<sup>2</sup> retail @ 2vtph per 100m<sup>2</sup> 25vtph (20 in / 5 out)
Total 68vtph (42 in / 26 out)
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Evening Peak Period

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Service Station (0.04 \times 830) + (0.3 \times 175) 86vtph (43 in / 43 out)
1,250m<sup>2</sup> retail @ 2vtph per 100m<sup>2</sup> 25vtph (5 in / 20 out)
Total 111vtph (48 in / 63 out)
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As can be seen, the traffic generating potential of the existing site development is significantly higher than the proposed traffic generating potential. However, in order to provide a conservative assessment, it has been assumed that the development site currently generates no traffic on the road network.



Based on current traffic flows on the road network, the following assignment of traffic has been adopted for this assessment:

- 40% to/from the west
- 40% to/from the east
- 20% to/from the north

Traffic Implications - Road Network Capacity

The main traffic implication of the proposed development in terms of road network capacity concerns the impact of traffic generated by the proposed development on the operating performance of the Waldron Road/Campbell Hill Road roundabout. That effect can be assessed using the SIDRA traffic model and criteria for interpreting the results of SIDRA analysis are set out on the schedule reproduced in the following pages.

The results of the SIDRA analysis of the operating performance of the Waldron Road/Campbell Hill Road intersection are set out on Table 3.1 and on the SIDRA MOVEMENT SUMMARY SHEETS reproduced in Appendix D revealing that the intersection will continue to operate satisfactorily with a high level of service and minimal delays.

TABLE 3.1 – RESULTS OF SIDRA ANALYSIS OF THE WALDRON ROAD / CAMPBELL HILL ROAD INTERSECTION

	Level of Service	Degree of Saturation	Total Average Vehicle Delay (sec)
Existing AM Peak	A	0.693	7.6
Existing PM Peak	A	0.690	7.2
Proposed AM Peak	A	0.711	7.9
Proposed PM Peak	A	0.713	7.6

In the circumstances, it can be concluded that the proposed development has no unacceptable traffic implications in terms of road network capacity.



Traffic-related Environmental Effect

As the development site is located on the higher order road network, there will be minimal impact on residential streets serving the site. Furthermore the traffic generating potential of the proposed development is significantly lower than the existing site development.

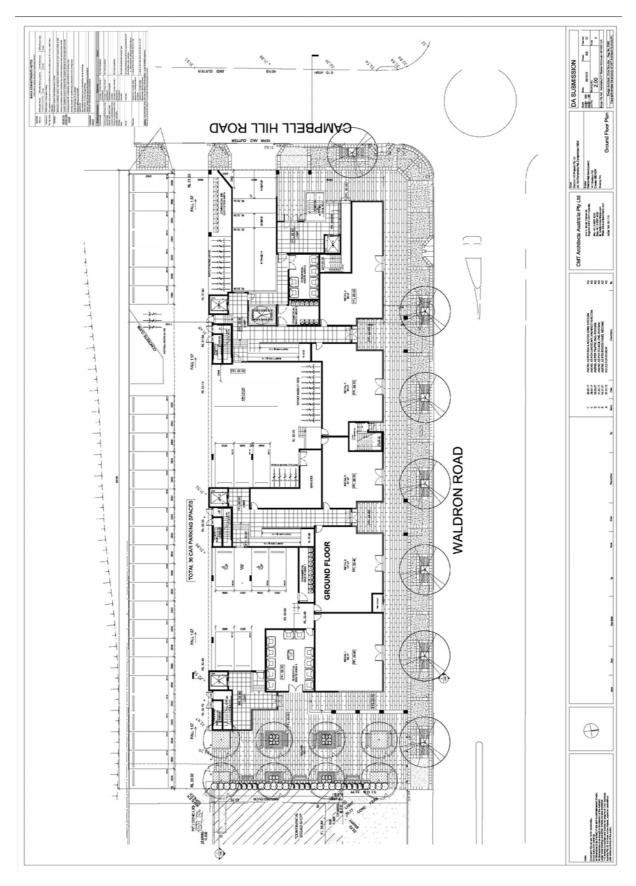
In the circumstances, the proposed development will not have any unacceptable traffic implications in terms of traffic-related environmental effect.



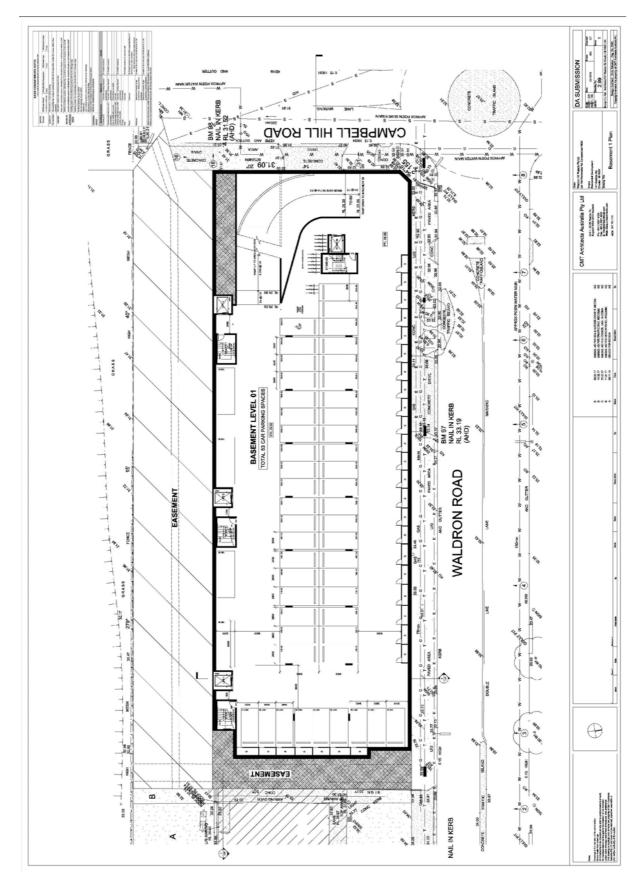
APPENDIX A

PLANS OF PROPOSED DEVELOPMENT PREPARED BY CMT ARCHITECTS

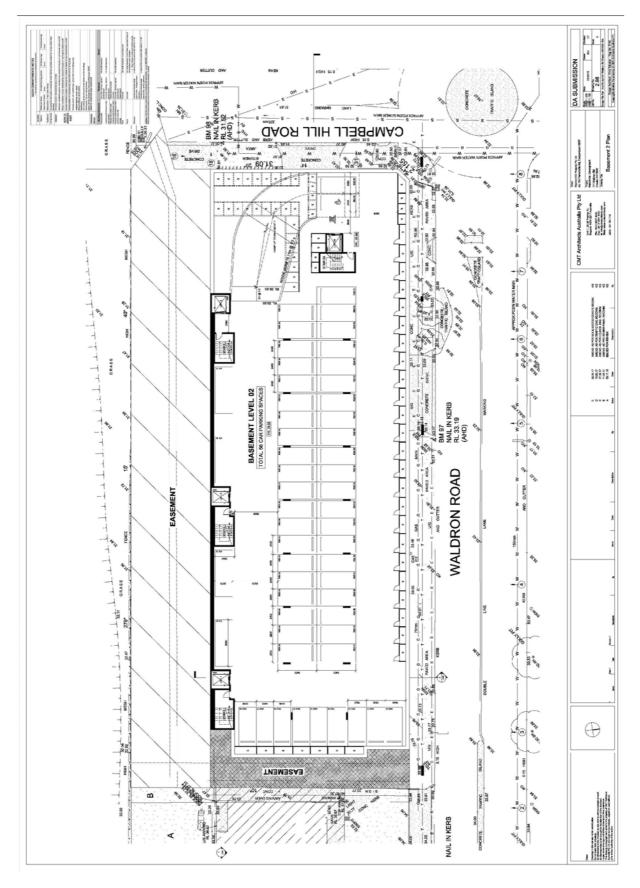








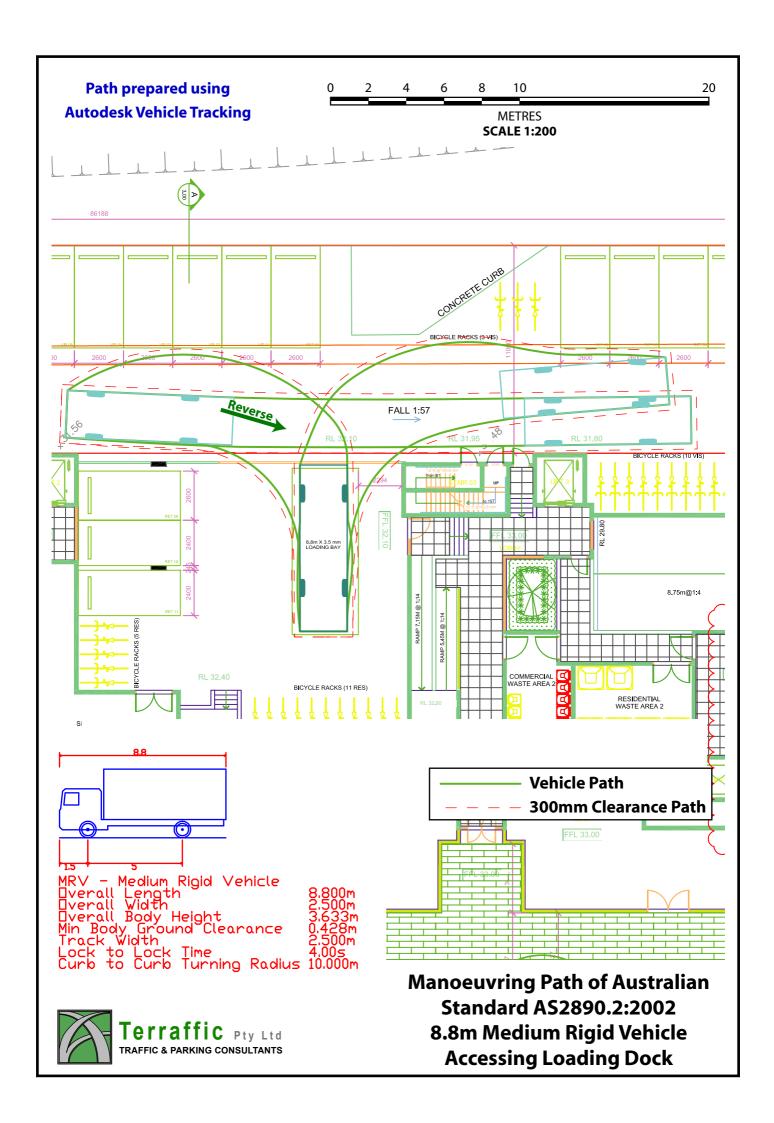


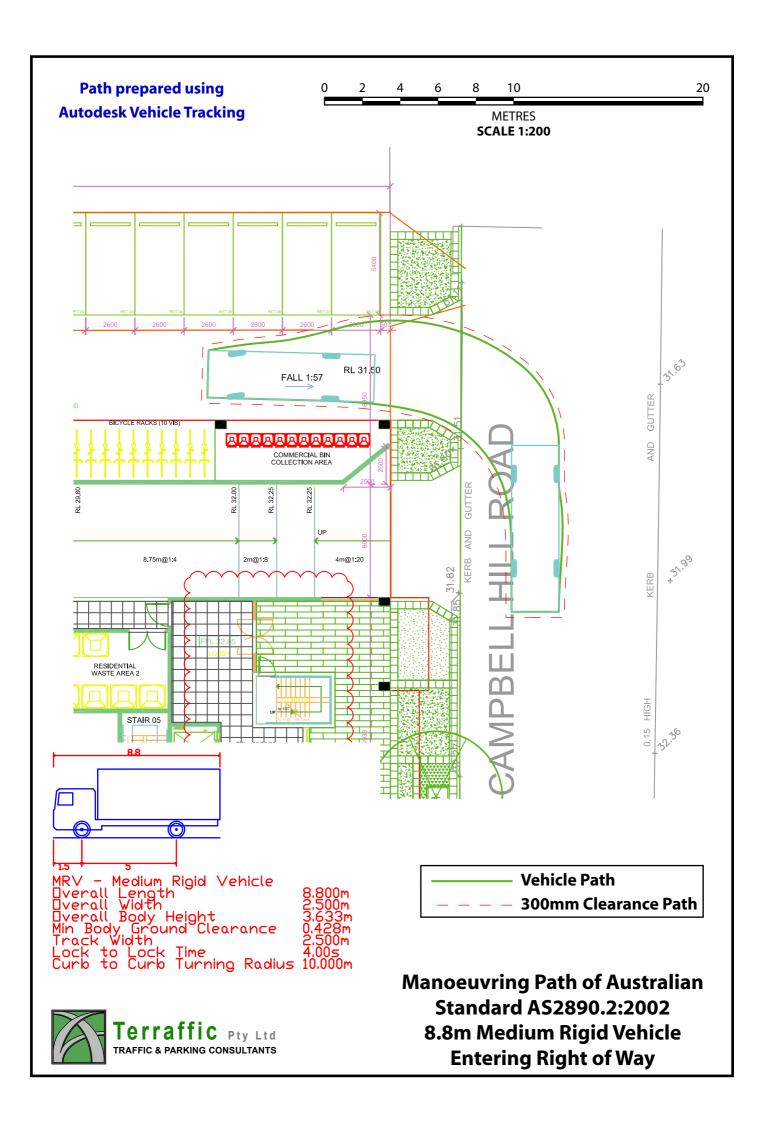


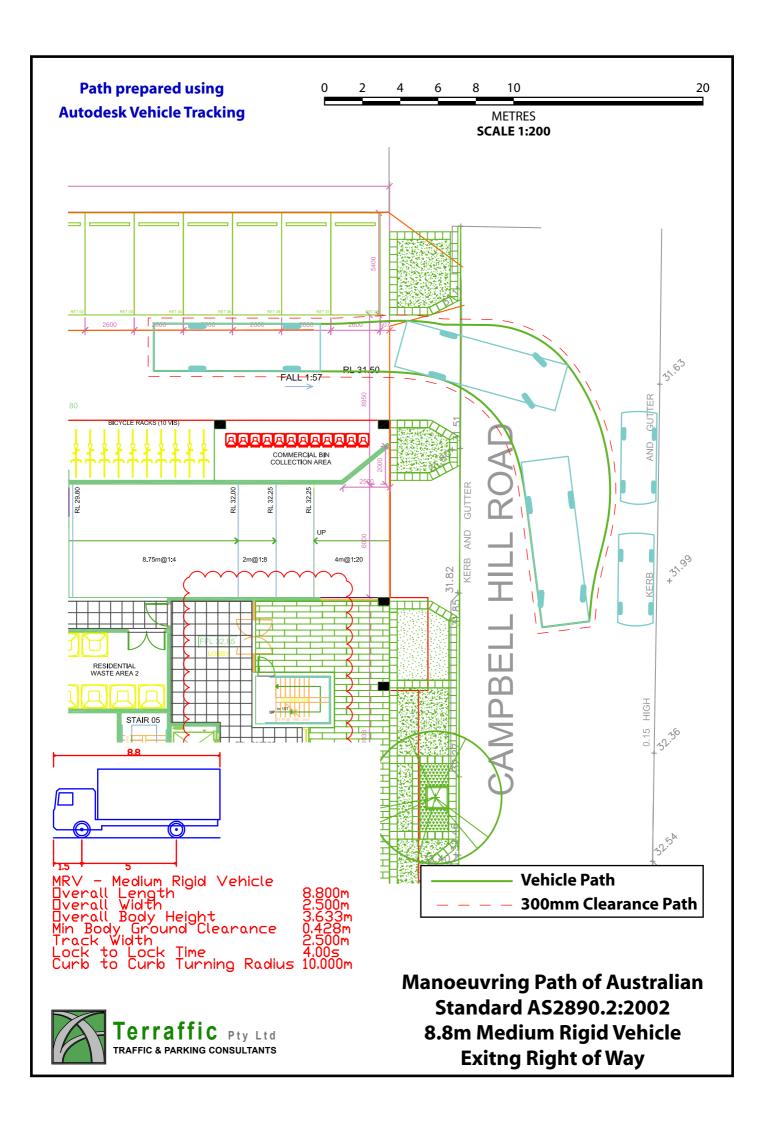


APPENDIX B

MEDIUM RIGID VEHICLE TURNING PATH DIAGRAM









APPENDIX C

TRAFFIC COUNT DATA



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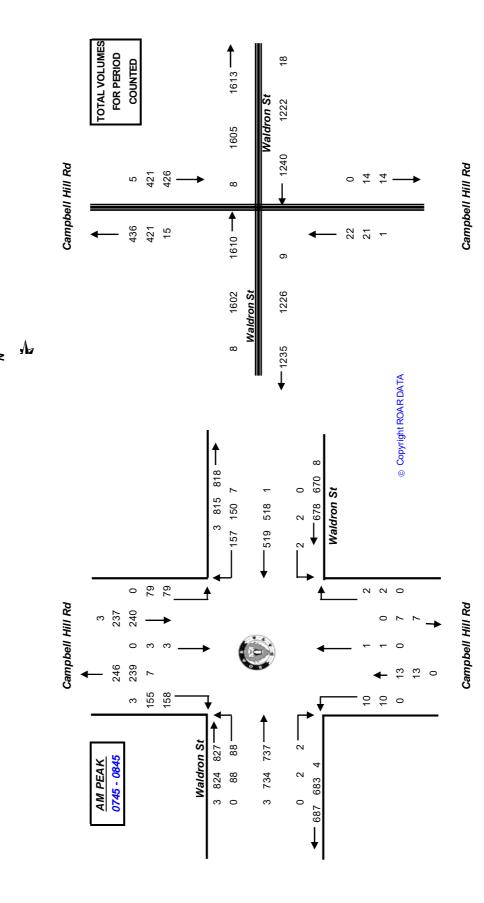
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Reliable, Original & Authentic Results
Ph.88196849, Mob. 0418 239019

Client : Terraffic Pty. Ltd.

Job No/Name : 5612 CHESTER HILL Waldron Rd

Day/Date: Wednesday 13th May 2015





R.O.A.R. DATA Reliable, Original & Authentic Results Ph.88196847, Fax 88196849, Mob. 0418 239019

Client : Terraffic Pty. Ltd.
Job No/Name : 5612 CHESTER HILL Waldron Rd
Day/Date : Wednesday 13th May 2015

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Job No/Name: 5612 CHESTER HILL Waldron Rd

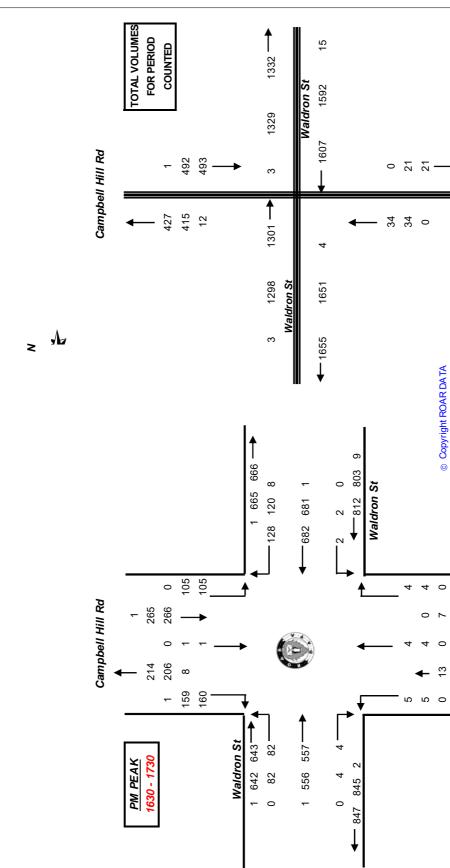
Client : Terraffic Pty. Ltd.

Day/Date: Wednesday 13th May 2015

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Ph.88196847, Fax 88196849, Mob. 0418 239019





Job No/Name: 5612 CHESTER HILL Waldron Rd

: Terraffic Pty. Ltd.

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R.O.A.R. DATA
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Job No/Name: 5612 CHESTER HILL Waldron Rd

Client : Terraffic Pty. Ltd.

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APPENDIX D

SIDRA MOVEMENT SUMMARY SHEETS



Site: Waldron Road and Campbell Hill Road, Chester Hill - Existing AM Peak

Existing AM Peak (7.45-8.45am) Roundabout

Mov	OD	Demand		Deg.	Average	Level of	95% Back		Prop.	Effective	Average
ID	Mov	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
South	: Campbell H	veh/h	%	v/c	sec		veh	m		per veh	km/t
30uur 1	. Campueir L2	10	0.0	0.023	10.2	LOSA	0.1	1.0	0.77	0.70	49.7
2	T1	1	0.0	0.023	10.3	LOSA	0.1	1.0	0.77	0.70	50.
3	R2	2	0.0	0.023	13.5	LOSA	0.1	1.0	0.77	0.70	50.2
Appro	ach	13	0.0	0.023	10.7	LOSA	0.1	1.0	0.77	0.70	49.8
East: \	Waldron Roa	nd									
4	L2	2	0.0	0.586	6.1	LOSA	5.7	40.1	0.62	0.61	51.6
5	T1	519	0.2	0.586	6.1	LOSA	5.7	40.1	0.62	0.61	52.4
6	R2	157	4.5	0.586	9.4	LOSA	5.7	40.1	0.62	0.61	51.
Appro	ach	678	1.2	0.586	6.9	LOSA	5.7	40.1	0.62	0.61	52.
North:	Campbell H	ill Road									
7	L2	79	0.0	0.407	11.0	LOSA	2.9	20.8	0.87	0.94	48.
8	T1	3	0.0	0.407	11.1	LOSA	2.9	20.8	0.87	0.94	48.9
9	R2	158	1.9	0.407	14.3	LOSA	2.9	20.8	0.87	0.94	48.6
Appro	ach	240	1.3	0.407	13.2	LOSA	2.9	20.8	0.87	0.94	48.
West:	Waldron Ro	ad									
10	L2	88	0.0	0.693	6.4	LOSA	7.6	53.5	0.69	0.60	51.
11	T1	737	0.4	0.693	6.4	LOSA	7.6	53.5	0.69	0.60	52.
12	R2	2	0.0	0.693	9.7	LOSA	7.6	53.5	0.69	0.60	52.
Appro	ach	827	0.4	0.693	6.4	LOSA	7.6	53.5	0.69	0.60	52.
All Vel	hicles	1758	0.8	0.693	7.6	LOSA	7.6	53.5	0.69	0.65	51.

Level of Service (LOS) Method: Delay (RTANSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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♥ Site: Waldron Road and Campbell Hill Road, Chester Hill - Existing PM Peak

Existing PM Peak (4.30-5.30pm) Roundabout

Mov	OD	Demand		Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Averag
ID	Mov	Total	H∀	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0 (0 1 11	veh/h	%	v/c	sec		veh	m		per veh	km/
	Campbell I		0.0	0.000	10.1	1.00.4					
1	L2	5	0.0	0.029	12.1	LOSA	0.2	1.3	0.85	0.75	48.
2	T1	4	0.0	0.029	12.2	LOSA	0.2	1.3	0.85	0.75	48.
3	R2	4	0.0	0.029	15.4	LOS B	0.2	1.3	0.85	0.75	48.
Approa	ach	13	0.0	0.029	13.2	LOSA	0.2	1.3	0.85	0.75	48.
East: \	Valdron Roa	ad									
4	L2	2	0.0	0.690	6.4	LOSA	7.7	54.1	0.71	0.62	51.
5	T1	682	0.1	0.690	6.4	LOSA	7.7	54.1	0.71	0.62	52.
6	R2	128	6.3	0.690	9.8	LOSA	7.7	54.1	0.71	0.62	51.
Approa	ach	812	1.1	0.690	7.0	LOSA	7.7	54.1	0.71	0.62	52.
North:	Campbell F	lill Road									
7	L2	105	0.0	0.354	8.7	LOSA	2.3	16.2	0.75	0.83	49.
8	T1	1	0.0	0.354	8.8	LOSA	2.3	16.2	0.75	0.83	50.
9	R2	160	0.6	0.354	12.0	LOSA	2.3	16.2	0.75	0.83	50.
Approa	ach	266	0.4	0.354	10.7	LOSA	2.3	16.2	0.75	0.83	50.
West:	Waldron Ro	ad									
10	L2	82	0.0	0.531	5.8	LOSA	4.7	33.1	0.52	0.55	52.
11	T1	557	0.2	0.531	5.8	LOSA	4.7	33.1	0.52	0.55	53.
12	R2	4	0.0	0.531	9.1	LOSA	4.7	33.1	0.52	0.55	52.
Approa	ach	643	0.2	0.531	5.8	LOSA	4.7	33.1	0.52	0.55	53.
All Vel	nicles	1734	0.6	0.690	7.2	LOSA	7.7	54.1	0.65	0.63	52.

Level of Service (LOS) Method: Delay (RTANSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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♥ Site: Waldron Road and Campbell Hill Road, Chester Hill - Proposed AM Peak

Proposed AM Peak (7.45-8.45am) Roundabout

Mov	OD	Demand		Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Averag
ID	Mov	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0 (0 1 11	veh/h	%	v/c	sec		veh	m		per veh	km/
	Campbell H		0.0	0.000	40.0	1.00.4			0.70	0 70	
1	L2	20	0.0	0.068	10.5	LOSA	0.4	2.9	0.79	0.78	49.
2	T1	6	0.0	0.068	10.5	LOSA	0.4	2.9	0.79	0.78	50.
3	R2	12	0.0	0.068	13.8	LOSA	0.4	2.9	0.79	0.78	49.
Approa	ach	38	0.0	0.068	11.5	LOSA	0.4	2.9	0.79	0.78	49.
East: \	Valdron Roa	ad									
4	L2	8	0.0	0.598	6.2	LOSA	5.8	41.1	0.64	0.62	51.
5	T1	519	0.2	0.598	6.3	LOSA	5.8	41.1	0.64	0.62	52.
6	R2	157	4.5	0.598	9.5	LOSA	5.8	41.1	0.64	0.62	51.
Approa	ach	684	1.2	0.598	7.0	LOSA	5.8	41.1	0.64	0.62	52.
North:	Campbell F	fill Road									
7	L2	79	0.0	0.424	11.5	LOSA	3.1	22.3	0.89	0.96	47.
8	T1	6	0.0	0.424	11.6	LOSA	3.1	22.3	0.89	0.96	48.
9	R2	158	1.9	0.424	14.8	LOSB	3.1	22.3	0.89	0.96	48.
Approx	ach	243	1.2	0.424	13.7	LOSA	3.1	22.3	0.89	0.96	48.
West:	Waldron Ro	ad									
10	L2	88	0.0	0.711	6.7	LOSA	8.1	56.6	0.73	0.63	51.
11	T1	737	0.4	0.711	6.8	LOSA	8.1	56.6	0.73	0.63	52.
12	R2	8	0.0	0.711	10.0	LOSA	8.1	56.6	0.73	0.63	52.
Approa	ach	833	0.4	0.711	6.8	LOSA	8.1	56.6	0.73	0.63	52.
All Vel	nicles	1798	0.8	0.711	7.9	LOSA	8.1	56.6	0.72	0.68	51.

Level of Service (LOS) Method: Delay (RTANSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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♥ Site: Waldron Road and Campbell Hill Road, Chester Hill - Proposed PM Peak

Proposed PM Peak (4.30-5.30pm) Roundabout

Mov	OD	Demand		Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
ID	Mov	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0 (0 1 11	veh/h	%	v/c	sec		veh	m		per veh	km/l
	Campbell H		0.0	0.000	10.1	1.00.4					
1	L2	14	0.0	0.080	12.4	LOSA	0.5	3.6	0.87	0.82	47.
2	T1	9	0.0	0.080	12.5	LOSA	0.5	3.6	0.87	0.82	48.
3	R2	13	0.0	0.080	15.7	LOS B	0.5	3.6	0.87	0.82	48.
Approa	ach	36	0.0	0.080	13.6	LOSA	0.5	3.6	0.87	0.82	48.
East: \	Valdron Roa	ad									
4	L2	12	0.0	0.713	6.9	LOSA	8.3	58.8	0.75	0.65	51.
5	T1	682	0.1	0.713	6.9	LOSA	8.3	58.8	0.75	0.65	52.
6	R2	128	6.3	0.713	10.2	LOSA	8.3	58.8	0.75	0.65	51.
Approa	ach	822	1.1	0.713	7.4	LOSA	8.3	58.8	0.75	0.65	51.
North:	Campbell F	fill Road									
7	L2	105	0.0	0.371	8.9	LOSA	2.4	17.2	0.77	0.85	49.
8	T1	7	0.0	0.371	9.0	LOSA	2.4	17.2	0.77	0.85	50.
9	R2	160	0.6	0.371	12.2	LOSA	2.4	17.2	0.77	0.85	50.
Approa	ach	272	0.4	0.371	10.9	LOSA	2.4	17.2	0.77	0.85	50.0
West:	Waldron Ro	ad									
10	L2	82	0.0	0.549	5.9	LOSA	4.9	34.4	0.55	0.57	52.
11	T1	557	0.2	0.549	6.0	LOSA	4.9	34.4	0.55	0.57	53.
12	R2	14	0.0	0.549	9.2	LOSA	4.9	34.4	0.55	0.57	52.
Approa	ach	653	0.2	0.549	6.0	LOSA	4.9	34.4	0.55	0.57	52.
All Vel	nicles	1783	0.6	0.713	7.6	LOSA	8.3	58.8	0.68	0.65	51.

Level of Service (LOS) Method: Delay (RTANSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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